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INDEX OF SHEETS

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2024 ROADWAY STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Contracts Standards and Development Unit -N. C. Department of Transportation - Raleigh, N. C., Dated January 16, 2024 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE DIVISION 2 - EARTHWORK Method of Clearing - Modified Method III Guide for Grading Subgrade - Secondary and Local Method of Obtaining Superelevation - Two Lane Pavement 200.03 225.02 225.04 Rock Plating 275.01

DIVISION 4 - MAJOR STRUCTURES 423.01 Bridge Approach Fills - Type 1 Approach Fill for Bridge Abutment

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS 846.01 Concrete Curb, Gutter and Curb & Gutter Guardrail Placement (Use Details in Lieu of Standards for Sheets 4, 6, 12, and 14 of 15) 862.01 Guardrail Installation 862.02 Structure Anchor Units (Use Detail in Lieu of Standard for Sheet 8 of 9) 862.03

GENERAL NOTES

EFFECTIVE: REVISED:

GRADE LINE: GRADING AND SURFACING:

> THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY MODIFIED METHOD III.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

TEMPORARY SHORING:

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC WILL BE PAID FOR AS "EXTRA WORK" IN ACCORDANCE WITH SECTION 104-7.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

AND LIST OF

SECTIONS

BRIDGE

FILL SECTION

BERM GUTTER SUMMARY, REMOVAL SUMMARY

ANS

SUMMARY

| | | PROJECT REFERENCE NO | . SHEET NO. |
|--|--|--|------------------------------------|
| | 555 Favetteville St. Suite 900 Raleigh, N.C. 27601 | DF18203.201080 | 0 1A |
| | N.C.B.E.L.S. License Number: F-0116 | ROADWAY DESIGN ENGINEER | |
| | | SEAL 040286 040280 040286 040280 040000000000 | |
| | | DOCUMENT NOT C UNLESS ALL SIGNA | ONSIDERED FINAL TURES COMPLETED |
| | | | |

EFF. 01-16-2024 REV.

01-16-2024

Note: Not to Scale

BOUNDARIES AND PROPERTY:

| State Line | |
|---|---|
| County Line | |
| Township Line | |
| City Line | |
| Reservation Line | · · |
| Property Line | |
| Existing Iron Pin (EIP) | — <u>·</u> |
| Computed Property Corner | – × |
| Existing Concrete Monument (ECM) | <u>F</u> |
| Parcel/Sequence Number | — (123) |
| Existing Fence Line | |
| Proposed Woven Wire Fence | |
| Proposed Chain Link Fence | |
| Proposed Barbed Wire Fence | |
| Existing Wetland Boundary | wLB |
| Proposed Wetland Boundary | |
| Existing Endangered Animal Roundary | FAB |
| Existing Endangered Animal Boundary | |
| Existing Endangered Flant Boundary | нрв |
| | |
| Rhown Confidmination Area: Soli | |
| Potential Confamination Area: Soil | — - <u>%</u> — » — <u>%</u> — » — |
| Known Contamination Area: Water | — - <u>22</u> , — w — <u>22</u> , — w — |
| Potential Contamination Area: Water | |
| Contaminated Site: Known or Potential | |
| | |
| BUILDINGS AND OTHER CULT | URE: |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap | <i>URE:</i> - 0 |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign | URE: - O - S |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well | <i>URE:</i> ♀ ♀ |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine | <i>URE:</i> ♀ ♀ ☆ |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation | <i>∙ v v v v v v v v v v</i> |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline | <i>URE:</i> |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery | <i>URE:</i> |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building | \square \bigcirc |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Sign Well Small Mine Foundation Area Outline Cemetery Building School | <i>URE:</i> - ○ - ♀ - ♀ - ↔ - ↓ - ↓ - ↓ |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church | - • • • • |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam | Image: With the second seco |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: | - • < |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water | - • < |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir | • • • |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream | URE: S |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 | • • • |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 | Image: Second state Image: Second state |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow | • • |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream | Image: Second |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream | \square \bigcirc \square |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland | \square \bigcirc \square |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland Proposed Lateral, Tail, Head Ditch | Image: Second secon |
| BUILDINGS AND OTHER CULT Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland Proposed Lateral, Tail, Head Ditch False Sump | $VRE:$ \odot \odot \odot \checkmark \checkmark \checkmark \checkmark \checkmark \Box \checkmark \blacksquare < |

Standard RR Signal Switch — RR Aband **RR** Dismantled

Primary H Primary H Secondar Vertical Be Existing R Proposed (Proposed Existing P Proposed Existing C Proposed Proposed Existing Ri Proposed Existing C Proposed Proposed Existing Ec Proposed Proposed Proposed Proposed Proposed Proposed Proposed

Existing Ed Existing C Proposed Proposed Proposed Existing M Proposed Existing Co Proposed Equality Sy Pavement VEGETA Single Tree Single Shr Hedge —

CONVENTIONAL PLAN SHEET SYMBOLS

RAILROADS:

| Gauge Milepost | CSX TRANSPORTATION O MILEPOST 35 |
|-------------------|--|
| loned | |

RIGHT OF WAY & PROJECT CONTROL:

| ••• //···· • •••• | | |
|--|----------------------------|---|
| Ioriz Control Point | | |
| Horiz and Vert Control Point | ۲ | N |
| y Horiz and Vert Control Point —— | \blacklozenge | |
| enchmark ——— | | |
| Right of Way Monument | \bigtriangleup | |
| Right of Way Monument ———— Rebar and Cap) | | |
| Right of Way Monument ———— Concrete) | | |
| ermanent Easement Monument —— | $\langle \cdot \rangle$ | |
| Permanent Easement Monument —— Rebar and Cap) | $\langle \diamond \rangle$ | |
| XA Monument ———— | \land | |
| C/A Monument (Rebar and Cap) — | ▲ | Ρ |
| C/A Monument (Concrete) ——— | | |
| ight of Way Line | | |
| Right of Way Line | | |
| Control of Access Line | | |
| Control of Access Line | | |
| ROW and CA Line ——— | | |
| asement Line | ——E—— | |
| Temporary Construction Easement- | E | |
| Temporary Drainage Easement —— | TDE | |
| Permanent Drainage Easement —— | PDE | |
| Permanent Drainage/Utility Easement | DUE | |
| Permanent Utility Easement | PUE | |
| Temporary Utility Easement | TUE | |
| Aerial Utility Easement | AUE | т |
| | | - |

ROADS AND RELATED FEATURES:

| dge of Pavement | |
|----------------------|--|
| Curb | |
| Slope Stakes Cut | <u>C</u> |
| Slope Stakes Fill | F |
| Curb Ramp ———— | CR |
| Aetal Guardrail ———— | <u> </u> |
| Guardrail ———— | <u> </u> |
| Cable Guiderail ———— | |
| Cable Guiderail | |
| ymbol | $igodoldsymbol{\Theta}$ |
| Removal — | $\times\!\!\times\!\!\times\!\!\times\!\!\times$ |
| ATION: | |
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| rub | ¢ |
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| Woods Line | |
|--|------------------|
| Orchard | - හි හි හි |
| Vineyard | - Vineyard |
| EXISTING STRUCTURES: | |
| MAJOR: | |
| Bridge, Tunnel or Box Culvert | CONC |
| Bridge Wing Wall, Head Wall and End Wall | -) CONC WW (|
| MINOR: | |
| Head and End Wall | CONC HW |
| Pipe Culvert | |
| Footbridge | > |
| Drainage Box: Catch Basin, DI or JB | Шсв |
| Paved Ditch Gutter | |
| Storm Sewer Manhole | (5) |
| Storm Sewer | S |
| <i>VIILIIES:</i> * SUE Subsurface Utility Engineering | |
| LOS – Level of Service – A.B.C or D | (Accuracv) |
| POWER: | (,,) |
| Existing Power Pole | - |
| Proposed Power Pole | - <mark>6</mark> |
| Existing Joint Use Pole | |
| Proposed Joint Use Pole | -6- |
| Power Manhole | - P |
| Power Line Tower | - |
| Power Transformer | - |
| U/G Power Cable Hand Hole | - H _H |
| H-Frame Pole | - • - • |
| U/G Power Line Test Hole (SUE – LOS A)* – | - |
| U/G Power Line (SUE – LOS B)* | P |
| U/G Power Line (SUE – LOS C)* | P P |
| U/G Power Line (SUE – LOS D)* | P |
| TELEPHONE: | |
| Existing Telephone Pole | · |
| Proposed Telephone Pole | - O- |
| Telephone Manhole | - ① |
| Telephone Pedestal | - |
| Telephone Cell Tower | - |
| U/G Telephone Cable Hand Hole | - H _H |
| U/G Telephone Test Hole (SUE – LOS A)* – | - |
| U/G Telephone Cable (SUE – LOS B)* | T |
| U/G Telephone Cable (SUE – LOS C)* | - <u> </u> |
| U/G Telephone Cable (SUE – LOS D)* | T |
| U/G Telephone Conduit (SUE – LOS B)* | - <u> </u> |
| U/G Telephone Conduit (SUE – LOS C)* | - <u> </u> |
| U/G Telephone Conduit (SUE – LOS D)* | TC |
| U/G Fiber Optics Cable (SUE – LOS B)* | - — — — T FO— - |
| U/G Fiber Optics Cable (SUE – LOS C)* | T FO |
| U/G Fiber Optics Cable (SUE – LOS D)* | TFO |

| | | PROJECT REFERENCE NO. DF18203.2010800 | sheet no. 1B |
|-------------------------|--|--|-----------------|
| | | | |
| | | | |
| | WATER: | | |
| | Water Manhole | W | |
| හි හි හි හි | Water Meter | O | |
| Vineyard | Water Valve | ─── ⊗ | |
| | Water Hydrant | ¢ | |
| | U/G Water Line Test Hole (SUE – LOS A | ∕)*— ↔ | |
| CONC | U/G Water Line (SUE – LOS B)* | w | |
|) CONC WW | U/G Water Line (SUE – LOS C)* | w | |
| | U/G Water Line (SUE – LOS D)* | ww | |
| CONC HW | Above Ground Water Line | | |
| | TV: | | |
| | TV Pedestal | | |
| СВ | | | |
| | U/G IV Cable Hand Hole | Ħ | |
| Ś | U/G TV Test Hole (SUE – LOS A)* | • | |
| S | $U/G IV Cable (SUE - LOS B)^*$ | | |
| | U/G TV Cable (SUE – LOS C)* | | |
| | U/G TV Cable (SUE – LOS D)* | TV | |
| Accuracyj | U/G Fiber Optic Cable (SUE – LOS B)* | TV FO_ | - — — |
| 4 | U/G Fiber Optic Cable (SUE – LOS C)* | —————————————————————————————————————— | |
| • | U/G Fiber Optic Cable (SUE – LOS D)* | TV FO | |
| | GAS: | ^ | |
| | Gas Valve | | |
| | Gas Meter | | |
| E M | U/G Gas Line Test Hole (SUE – LOS A) | * <u> </u> | |
| | U/G Gas Line (SUE – LOS B)* | | |
| ₩. | U/G Gas Line (SUE – LOS C)* | C | |
| | U/G Gas Line (SUE – LOS D)* | G | |
| •• | Above Ground Gas Line ———— | | |
| • • • • • • • • • • • • | SANITARY SEWER: | | |
| P | Sanitary Sewer Manhole | ® | |
| P | Sanitary Sewer Cleanout | ÷ | |
| | U/G Sanitary Sewer Line | ss | Sewer |
| | Above Ground Sanitary Sewer | | |
| • -~ | SS Force Main Line Test Hole (SUE – LO | JS A)* | |
| | SS Force Main Line (SUE $-$ LOS B) ⁺ $-$ | | |
| | SS Force Main Line (SUE $= LOS C$) = | | |
| L. | $\frac{33}{100} = \frac{301}{100} = \frac{103}{100} = \frac$ | FSS | |
| v ~ > ⊞J | MISCELLANEOUS: | | |
| <u>∩</u> | Ultility Polo with Base | U | |
| | Utility Located Object | | |
| T | Utility Traffic Signal Pay | O | |
| т | | [5] >* | |
| - — — — TC — — — – | UTILITY UNKNOWN U/G LINE (SUE – LOS E |) | |
| | U/G Tank; Water, Gas, Oil | | |
| | Underground Storage Tank, Approx. Loc. | (<u>UST</u>) | |
| | A/G Tank; Water, Gas, Oil | | |
| | Geoenvironmental Boring | | _ |
| T F0 | Abanaonea According to Utility Records | AATU | ĸ |
| | End of Information | —— E.O.I | • |



| TO | STATIC | DN | |
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| 14 + | -05.00 | (BEGIN | BRIDGE) |
| 16+ | 30.00 | · | |



8-0CT-2017 14:17 S:\Contracts\Contra ihowerton AT CSD

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| | | | - | | IN CUBIC YARDS | | | | | | | | | | | | | | | | | |
| | | | | | UNCLASSIFIED | FMRANK - 0/ | BORROW | WACTE | | | | | | | | | | | | | | |
| | | -I- STA 12+50.00 TO STA | 14+05.00 (BEGI | | EXCAVATION 64 | 150 | 86 | 0 | _ | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | |
| | | SUI | BTOTAL | | 64 | 150 | 86 | 0 | _ | | | | | | | | | | | | | |
| | | –L– STA. 14+62.25 (END BR | RIDGE) TO STA. 17 | 7+80.97 | 56 | 396 | 340 | 0 | | | | | | | | | | | | | | |
| | | SUI | BTOTAL | | 56 | 396 | 340 | 0 | | | | | | | | | | | | | | |
| | | | ΟΤΔΙ | | 120 | 514 | 104 | | | | | | | | | | | | | | | |
| | | MATERIAL FOR SHOU | | TION | 120 | 540 | 420 | 0 | - | | | | | | | | | | | | | |
| | | LOSS DUE TO CLEAR | RING & GRUBBIN | NG | | | | |] | | | | | | | | | | | | | |
| | | ROCK WASTE TO REI | PLACE BORROW | | | | | | | | | | | | SUMM | ARY | OF | | | | | |
| | | WASTE IN LIEU OF E | BORROW | | | | | | | | | | C | | FR RF | R M | CII 3 | TTF | R | | | |
| | | PROJECT | | | 120 | 546 | 426 | 0 | _ | | | | 3 | | | | | | | | | |
| | | ESI. 5% TO REPLACE TOP | SUIL ON BORR | COW PII | | | 21 | | | | | | | | IN | FEET | | | | | | |
| | | GRAN | ND TOTAL | | 120 | 546 | 447 | | 1 | | | | | LOC | | PE BEC STAT | GIN END TION STATIO | | LENGTH | | | |
| | | | SAY | | 150 | | 500 | | | | | | | | | | 70 | | | | | |
| | | Note: Approximate | quantities or | nly. Unclassified F | xcavation. Borro | w Excavati | on. Fine (| Gradina | | | | | | | -L- LT -L- R1 | 13 + 13 + | - 78 13+94 - 78 13+94 | 4 4 | 16 16 | | | |
| | | Clearing and Grubb | bing, and Re | emoval of Existing | Pavement will b | e paid for | at the | 2 | | | | | | | | | | | | | | |
| | | contract lump sum | price for "G | arading." | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | ТО | TAL | 32 | | | |
| | | ESTIMATED UNDERCUT = 4 ESTIMATED GEOTEXTILE FOR | SOIL STABILIZATI | ION = 200 SY | | | | | | | | | | | | | S | AY | 40 | | | |
| | | ESTIMATED SELECT GRANULA | AR MATERIAL, CLA | 455 III = 200 CY | | | | | | | | | | | | | | | | | | |
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| | | | | | | | | | | | | | | | | | | | | | | |
| "N" = DIS | ISTANCE FROM EDGE C | DE LANE TO FACE OF GUARDRAII | | | | | | | | | | | | | | | | | | | | |
| "N" = DIS TOTAL SHOU FLARE LENG | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRC | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA | SHOULDER BREA | AK POINT. GUARDRAIL. | | | | | | | | | | | | | | | | | | |
| $\begin{array}{l} "N" = DIS\\ TOTAL SHOWFLARE LENG\\ W = TOT\\ G = GAT\\ \end{array}$ | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRC DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O JTOR TYPE 350 | SHOULDER BREA AIL TO END OF OF GUARDRAIL. | AK POINT. GUARDRAIL. | | | | | | GUARD | RAIL S | SUMM | 4RY | | | | | | | | | |
| "N" = DIS TOTAL SHOU FLARE LENG W = TOT G = GAT NG = NO | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRC DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA NON-GATING IMPACT A | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O ITOR TYPE 350 ATTENUATOR TYPE 350 | SHOULDER BREA AIL TO END OF DF GUARDRAIL. | AK POINT. GUARDRAIL. | | DANIT DOINT | | ″N″ | | GUARD. | RAIL S | SUMM | 4RY | | NCHORS | | | | ACT | | REMOVE | |
| "N" = DISTOTAL SHOUFLARE LENGW = TOTG = GATNG = NCSURVEYLINE | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRO DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA NON-GATING IMPACT A BEG. STA. | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O TOR TYPE 350 ATTENUATOR TYPE 350 END STA. LOCATION | SHOULDER BREA AIL TO END OF DF GUARDRAIL. | AK POINT. GUARDRAIL. LENGTH | WA | RANT POINT | | "N" T DIST. T FROM S | TOTAL SHOUL. | GUARD. FLARE LENGTH | RAIL S | SUMM. | 4RY | A | NCHORS | | | | ACT UATOR 350 FACED | REMOVE | REMOVE AND STOCKPILE | REMARKS |
| "N" = DIS TOTAL SHO FLARE LENG W = TOT G = GAT NG = NC SURVEY LINE | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRC DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA NON-GATING IMPACT A BEG. STA. | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O ITOR TYPE 350 ATTENUATOR TYPE 350 END STA. LOCATION | SHOULDER BREA AIL TO END OF OF GUARDRAIL. | AK POINT. GUARDRAIL. LENGTH SHOP DOUBLE FACED | WAI APPROACH END | RANT POINT TRAILING END | G F | "N" T DIST. S FROM V E.O.L. V | TOTAL SHOUL. WIDTH A | GUARD. FLARE LENGTH PROACH TRAILIN END TRAILIN | RAIL S | S UMM W TRAILING END | 4RY XI AT-1 | Al GREU, TYPE TL-3 III | NCHORS B-77 CAT- | -1 B–77 SHOP CURVED | XIII XI | IMPA ATTENU TYPE EA G | ACT UATOR 350 GUARDRAIL 3 NG | REMOVE EXISTING GUARDRAIL | REMOVE AND STOCKPILE EXISTING GUARDRAIL | REMARKS |
| "N" = DIS TOTAL SHO FLARE LENG W = TOT G = GAT NG = NC SURVEY LINE -L- | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRO DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA NON-GATING IMPACT A BEG. STA. 12+74 | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O TOR TYPE 350 ATTENUATOR TYPE 350 END STA. LOCATION 14+05 (BR) RT | SHOULDER BREA AIL TO END OF OF GUARDRAIL. STRAIGHT 131.25 | AK POINT. GUARDRAIL. LENGTH SHOP DOUBLE FACED | WAI APPROACH END 13 + 70 | RANT POINT TRAILIN END 14+05 | G F | "N" T DIST. S FROM V E.O.L. V | TOTAL SHOUL. WIDTH AI | GUARD. FLARE LENGTH PROACH TRAILIN END TRAILIN END | RAIL S | S UMM W TRAILING END | А.R.Y хі ат-1 | GREU, TL-3 TYPE III 1 1 | NCHORS B-77 CAT- | 1 B–77 SHOP CURVED | XIII XI | | ACT UATOR 350 3 NG S NG | REMOVE EXISTING GUARDRAIL | REMOVE AND STOCKPILE EXISTING GUARDRAIL | REMARKS 8' GUARDRAIL POSTS |
| "N" = DISTOTAL SHOFLARE LENGW = TOTG = GATNG = NCSURVEYLINE-LL- | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRO DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA NON-GATING IMPACT A BEG. STA. 12+74 12+99 | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O TOR TYPE 350 ATTENUATOR TYPE 350 END STA. LOCATION 14+05 (BR) RT 14+05 (BR) LT | SHOULDER BREA AIL TO END OF OF GUARDRAIL. STRAIGHT 131.25 106.25 | AK POINT. GUARDRAIL. LENGTH SHOP DOUBLE FACED | WAI APPROACH END 13 + 70 12 + 50 | RANT POINT TRAILIN END 14+05 14+05 | G F | "N" T DIST. S FROM V 4' 4' | TOTAL SHOUL. WIDTH AI 4' 4 | GUARD. FLARE LENGTH PROACH TRAILIN END TRAILIN END 112.5' 87.5' | RAIL S IG APPROACH END 1.58' 1.35' | SUMMA W TRAILING END | XI AT-1 MOD I | GREU, TL-3 TYPE III 1 1 1 1 | NCHORS B-77 CAT- | 1 B–77 SHOP CURVED | XIII XI | | ACT UATOR 350 SINGLE FACED GUARDRAIL 3 NG | REMOVE EXISTING GUARDRAIL | REMOVE AND STOCKPILE EXISTING GUARDRAIL | REMARKS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS |
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| "N" = DISTOTAL SHOFLARE LENCW = TOIG = GATNG = N(SURVEYLINE $-L-$ $-L-$ $-L-$ $-L-$ | ISTANCE FROM EDGE C OULDER WIDTH = DIS IGTH = DISTANCE FRC DTAL WIDTH OF FLARE F ATING IMPACT ATTENUA NON-GATING IMPACT A BEG. STA. 12+74 12+99 14+62 (BR) 14+62 (BR) | DF LANE TO FACE OF GUARDRAIL. TANCE FROM EDGE OF TRAVEL LANE TO DM LAST SECTION OF PARALLEL GUARDRA FROM BEGINNING OF TAPER TO END O TOR TYPE 350 ATTENUATOR TYPE 350 TITENUATOR TYPE 350 END STA. LOCATION 14+05 (BR) RT 14+05 (BR) LT 16+93 RT 17+81 LT 16+93 RT 17+81 LT SUBTOTAL GREU, TL-3 @ 50'X4 TYPE III @ 18.75'X4 | SHOULDER BREA ALL TO END OF DF GUARDRAIL. STRAIGHT 131.25 106.25 231.25 318.75 318.75 787.50 -200 -75 | AK POINT. GUARDRAIL. LENGTH SHOP DOUBLE FACED SHOP I DOUBLE FACED | WAI APPROACH END 13+70 12+50 14+62 14+62 14+62 | RANT POINT TRAILIN END 14+05 14+05 16+00 16+57 | G F | "N" T DIST. S FROM V 4' 4' 4' 4' | TOTAL SHOUL. WIDTH 4' | GUARD. FLARE LENGTH PROACH END 112.5' 87.5' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' 29' 50' | RAIL S IG APPROACH END 1.58' 1.35' 58' 58' 58' | V TRAILING END | XI AT-1 MOD I I I <tr< td=""><td>A</td><td>NCHORS B-77 CAT- 1</td><td>B-77 SHOP CURVED I <tr< td=""><td>XIII XI </td><td>EA G</td><td>ACT UATOR 350 SINGLE FACED GUARDRAIL 3 NG 4 III 4 III 5 IIII 5 IIII 5 III 5 III 5 II</td><td>REMOVE EXISTING GUARDRAIL</td><td>REMOVE AND STOCKPILE EXISTING GUARDRAIL</td><td>REMARKS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS</td></tr<></td></tr<> | A | NCHORS B-77 CAT- 1 | B-77 SHOP CURVED I <tr< td=""><td>XIII XI </td><td>EA G</td><td>ACT UATOR 350 SINGLE FACED GUARDRAIL 3 NG 4 III 4 III 5 IIII 5 IIII 5 III 5 III 5 II</td><td>REMOVE EXISTING GUARDRAIL</td><td>REMOVE AND STOCKPILE EXISTING GUARDRAIL</td><td>REMARKS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS</td></tr<> | XIII XI | EA G | ACT UATOR 350 SINGLE FACED GUARDRAIL 3 NG 4 III 4 III 5 IIII 5 IIII 5 III 5 III 5 II | REMOVE EXISTING GUARDRAIL | REMOVE AND STOCKPILE EXISTING GUARDRAIL | REMARKS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS 8' GUARDRAIL POSTS |
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STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

| LOCATION | SIDE | BEGIN STATION | END STATION | LENGTH |
|----------|------|------------------|----------------|--------|
| | | | | |
| -L- | LT | 13 + 78 | 13+94 | 16 |
| -L- | RT | 13 + 78 | 13+94 | 16 |
| | | | | |
| | | | | |
| | | | | |
| | | | TOTAL | 32 |
| | | | SAY | 40 |

| PROJECT RE | FERENCE NO. | SHEET NO. |
|------------|---|--|
| DF1820 | 3.2010800 | 3B–1 |
| FSS | HDR Engineering, 555 Fayetteville St, Suite N.C.B.E.L.S. Lice | Inc. of the Carolinas 900 Raleigh, N.C. 27601 nse Number: F-0116 |

AM C DRIVER: NCDOT. CHARRIS NCDOT\NCDOT_ LOT SER: ILE:

| TUM DESCRIPTION | Prepared in the Office of: | | | | | |
|---|--|--|--|--|--|--|
| ORDINATE SYSTEM DEVELOPED FOR THIS PROJECT STATE PLANE COORDINATES ESTABLISHED BY NCGS FOR MONUMENT "GPS-2" A 2011 STATE PLANE GRID COORDINATES OF G: 69023.1582(ft) EASTING: 2301550.3653(ft) ELEVATION: 14.758(ft) OMBINED GRID FACTOR USED ON THIS PROJECT ROUND TO GRID) IS: 1.0001390365 N.C. LAMBERT GRID BEARING AND | LOCATION AND 5310 BARBADOS CASTLE HAYNE, NOI | O SURVEYS UNIT BLVD., SUITE 102 RTH CAROLINA 28409 | | | | |
| HORIZONTAL GROUND DISTANCE FROM | 2024 STANDARD SPE | ECIFICATIONS | | | | |
| PS-2" TO -L- STATION 10+00.00 IS N 24-47'40.1" E 206.63(ft) | RIGHT OF WAY DATE: | LETTING DATE | | | | |
| SIONS ARE LOCALIZED HORIZONTAL DISTANCES RTICAL DATUM USED IS NAVD 88 | N⁄A | NOVEMBER 7, 20 | | | | |

| EL | | | | |
|-------|-----------|-------------|-----------------|---------|
| POINT | N | | BEARING | DIST |
| POT | 68973.896 | 23Ø1548.536 | | |
| LINE | | | N 20°29′06.9" E | 1200.00 |
| POT | 70098.011 | 23Ø1968.496 | | |

| BL | | | | |
|------|-----------------|------------|--------------|-----------|
| POI | NT DESC. | NORTH | EAST | ELEVATION |
| | | | | |
| GPS1 | GPS PK | 68116.7486 | 2301239.9631 | 21.72 |
| GPS2 | GPS CAP & REBAR | 69023.1582 | 2301550.3653 | 14.76 |
| BL1 | TRV CAP & REBAR | 69570.6660 | 2301786.4780 | 9.28 |
| BL2 | TRV CAP & REBAR | 70229.5280 | 2302033.1220 | 15.57 |

SURVEY CONTROL SHEET

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

I, Christopher J. Sawyer, PLS, certify that the Project Control was performed under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

Class of survey: **AA** Type of GPS field procedure: RTN Dates of survey: OCTOBER 17, 2024 Datum/Epoch: NAD 83/ NA 2011 Published/Fixed-control use: N/A FOR RTN Localized around: GPS-2 Northing: 69023.1582 Easting: 2301550.3653 Combined grid factor: 1.0001390365 Geoid model: G18NC Units: US SURVEY FEET

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed on October 17, 2024, and all coordinates are based on NAD 83/ NA2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 24th day of October, 2024.

Christopher Bawyer Professional Land Surveyor L-4526

2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

BL-2

Location and Surveys LOCATION AND SURVEYS UNIT 5310 BARBADOS BLVD., SUITE 102 CASTLE HAYNE, NORTH CAROLINA 28409 PROJECT SURVEYOR SEAL L-4526 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO.

RW02C-1

PROJECT REFERENCE NO.

DF18203.2010800

| б С | | | | |
|---|---|--|---|--|
| ^{Projects} \BRIDGE\Brunswick\Culvert over Price's Creek\Working\Control Sheets\DF18203.2010800_LS_rw02d-1_241023.dgn -328/25L | | | | |
| uval AT LS-328725L uval AT LS-328725L | | | | |
| | Units/Div03?Projects/BRIDGE/Brunswick/Culvert over Price's Creek/Working/Control Sheets/DF18203.2010800_LS.rw02d-1_241023.dgn | With Transmith Converted Street North Ing Control Streets OF 123.030 (2012) - 422-124123.030 (2012) - 422-124123.030 (2012) - 422-124123.030 (2012) - 422-12423.030 (201 | int 1012: Thurmanek Curver Lover Prees Great Mag 1.2 423 1.2.1.823431 | Comparison (1996) (2000) Comparison |

PROPOSED ALIGNMENT CONTROL SHEET

| STATION | NORTH | EAST |
|----------|------------|--------------|
| 10+00.00 | 69210.7450 | 2301637.0206 |
| 18+48.47 | 70005.5602 | 23Ø1933.9568 |

NOTES:

- THE LOCATION AND SURVEYS UNIT.

I, Christopher J. Sawyer, PLS, certify that the data compiled came from available surveys/mapping performed by others and provided to me by NCDOT and do not certify to the accuracy or quality of the individual data sources.

This 24th day of October, 2024.

Christopher Bawyer

Professional Land Surveyor L-4526

1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.

2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT

| NDARD SPECIFICA | ATIONS |
|-------------------------------|-----------------------------|
| Designed by: Tagner II, PE | 4286 |
| 'a | gner II, PE Level III ce |

EROSION & SEDIMENT CONTROL LEGEND

| <u>Std. #</u> | Description |
|---------------|-------------------------------------|
| 1605.01 | Temporary Silt Fence |
| 1606.01 | Special Sediment Control Fence |
| 1622.01 | Temporary Berms and Slope Drain |
| 1630.02 | Silt Basin Type B |
| 1630.03 | Temporary Silt Ditch |
| 1630.04 | Stilling Basin |
| 1630.05 | Temporary Diversion |
| 1630.06 | Special Stilling Basin |
| 1630.07 | Skimmer Basin |
| 1630.08 | Tiered Skimmer Basin |
| 1630.09 | Earthen Dam with Skimmer |
| | Infiltration Basin |
| 1632.01 | Rock Inlet Sediment Trap: Type A |
| 1632.02 | Туре В |
| 1632.03 | Туре С |

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

| PROJECT REFERENC | SHEET NO. | |
|----------------------------|-----------|------------------------|
| DF18203.201080 | EC-02 | |
| | | |
| ROADWAY DESIGN ENGINEER | | HYDRAULICS ENGINEER |

Symbol

Temporary Rock Silt Check Type A

- Temporary Rock Silt Check Type B
- Temporary Rock Silt Check Type A with **Excelsior Matting and Flocculant**
- Temporary Rock Sediment Dam Type A 1000 2000 1000 2000
- Temporary Rock Sediment Dam Type B
- Rock Pipe Inlet Sediment Trap Type A
- --B €€ Rock Pipe Inlet Sediment Trap Type B
- Excelsior Wattle Check with Flocculant

 - **EW** -
- Silt Fence Coir Fiber Wattle Break +CFW+

---EW---EW---EW----

----CFW----CFW-----CFW-----

| NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD 2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD 3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. | | PROJECT REFERENCE NO. | SHEET NO. |
|--|--|--|------------------------|
| NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD 2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEEDÂRD. 3. CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. | | DF18203.2010800 | EC-02A |
| NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD 2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD. 3. CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. | | R/W SHEET NO. | |
| NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD 2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD 3. CONCRETE WASHOUT STRUCTURE-NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. | | COADWAY DESIGN ENGINEER | HYDRAULICS ENGINEER |
| NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD 2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD. 3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. TYP.) | | | |
| NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD 2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD. 3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. TYP.) | | | |
| TYP.) | CATION DETERMINE | | DE |
| 3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE. | VHEN THE LIQUID AN TRUCTURES CAPAC DEDING CAPACITY W REEBOARD | DOTORES SHALL D/OR SOLID REA TY TO PROVIDE ITH A MINIMUM 12 | DE CHES 2 |
| (TYP.) | WASHOUT STRUCTU (ED WITH SIGNAGE I | RE NEEDS TO BE IOTING DEVICE. | |
| (TYP.) | | | |
| (TYP.) | | | |
| & ATION | | | |

NOTES: 1. ACTUAL LOCATION DETERMINED IN FIELD

2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD.

3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE.

SILT FENCE COIR FIBER WATTLE BREAK

NOTES:

LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 11 GUAGE STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 6" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED. INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

INSET A

|--|

| PROJECT REFERENCE NO. | SHEET NO. |
|----------------------------|------------------------|
| DF18203.2010800 | EC-2B |
| R/W SHEET NO. | |
| ROADWAY DESIGN ENGINEER | HYDRAULICS ENGINEER |

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND

SIDE VIEW

COIR FIBER WATTLE BARRIER DETAIL

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED. DO NOT PLACE WATTLES ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL

CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.

INSET A

FILL SLOPE

TOP VIEW

PROJECT REFERENCE NO. SHEET NO. DF18203.2010800 EC-2C R/W SHEET NO. ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER

| | | | | ST. | DIVISION OF ATE OF NOF | ' HIGHWA RTH CAR | YS OLINA | FS | HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116 | PROJECT REFERENCE NO. DF 18203.2010800 ROADWAY DESIGN ENGINEER | SHEET NO. EC-3 HYDRAULICS ENGINEER |
|--------------------|--|---|---|--------------|--|---------------------|---------------------|-----------------|---|---|---|
| | | | SOIL | STA | BILIZATION | N SUMM | ARY SHEE | E T | | | |
| | COIR FIBER | MATTING | FOR S | SLOPE | | Ι | ATTING FOR | DITCH | IES | | |
| CONST SHEET NO. | LINE | FROM STATION | TO STATION | SIDE | ESTIMATE (SY) | CONST SHEET NO. | LINE | FROM STATION | TO STATION SIDE | ESTIMATE | (SY) |
| | - L - - L - - L - MATTING SCELLANEOUS AS | 3 + 0 0 3 + 0 0 5 + 0 0 FOR GLOF DIRECTED 1 1 1 1 1 1 1 1 1 1 | 3 + 75 6 + 25 6 + 25 2 E S SUB BY ENG | | 275 275 450 450 1450 GY 550 GY 2000 GY | | NOTE: NO DITCH MATT | | ATED ON THIS PROJECT ATED ON THIS PROJECT Image: Constraint of the second se | | |
| | | | | | | | | | | | |

SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 TO 4:1

ALL OTHER AREAS WITH SLOPES FLATTER

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

| | STABILIZATION TIME | 7/ |
|------------|--------------------|---------------------|
| SLOPES | 7 DAYS | NONE |
| | 7 DAYS | NONE |
| | 7 DAYS | IF SLOPE Not ste |
| | | 7 DAYS Length |
| | 14 DATS | 7 DAYS Perimete |
| R THAN 4:1 | 14 DAYS | 7 DAYS Perimete |
| | | |

| PROJECT REFERENCE NC | . SH | EET NO. |
|----------------------------|-----------------|---------------|
| DF18203.2010800 | C = E(| C-3A |
| | | |
| ROADWAY DESIGN ENGINEER | HYDRAI ENGIN | JLICS IEER |

IMEFRAME EXCEPTIONS

ES ARE IO'OR LESS IN LENGTH AND ARE EEPER THAN 2:1, 14 DAYS ARE ALLOWED.

FOR SLOPES GREATER THAN 50' IN WITH SLOPES STEEPER THAN 4:1.

FOR PERIMETER DIKES, SWALES, DITCHES ER SLOPES, AND HQW ZONES

FOR PERIMETER DIKES, SWALES, DITCHES ER SLOPES, AND HQW ZONES

| REVISIONS | |
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| | 8/17/99 |
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| | RYAN KINIF AND WIFE ZA MANRIQI B 4409 PG MB 19 PG 33 |
| WOODS | 2Y JE-KINIRY 615 |
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| | 8/17/ | | | | |
|-------|----------------------|--------------|---------------|--|-----------|
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| | 51 ⁴⁵¹¹⁶⁶ | | sta. 10+00.00 | | |
| | | | 5 10d -7- | RYAN KINIRY AND WIFE MARITZA MANRIQUE-KINIRY DB 4409 PG 615 MB 19 PG 337 | |
| | EIR GPS-2 | EXISTING R/W | | د <u>زن جن F0</u> | |
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| 03.20108 | SIGNING & PAVEMEN BRUNSWICK LOCATION: SR 1528 (E. MOORE STREET) OVE | IT MARKING PLAN COUNTY FR PRICE CREEK NORTH OF NC 211 | DATE: <u>10/24</u> SEAL DOCUM UNLESS |
|--------------|---|--|--|
| DF182 | INDEX SHEET NO. DESCRIPTION S&PMP-1 PLAN TITLE, GENERAL NOTES, ROADWAY STANDARD DRAWINGS, INDEX, AND SUMMARY OF QUANTITIES S&PMP-2 SIGNING & PAVEMENT MARKING PLAN | GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, OR DIRECTED BY THE ENGINEER. A) INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE AS FOLLOWS: <u>ROAD NAME</u> <u>MARKING</u> -L- THERMOPLASTIC POLYCARBONATE H-SHAPED | E |
| : DC00471 | ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - N.C. DEPARTMENT OF THANSPORTATION - RALEIGH, N.C., DATED JANUARY 2024 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS: STD. NO. ITILE 903.10 GROUND MOUNTED SIGN SUPPORTS 904.20 WOOD SIGN SUPPORTS 904.10 ORIENTATION OF GROUND MOUNTED SIGNS 904.50 MOUNTING OF TYPE 'D', 'E' AND 'F' SIGNS ON 'U' CHANNEL POSTS 1205.01 PAVEMENT MARKINGS - TWO-LAWE AND MULTI-LANE ROADWAYS 1205.02 PAVEMENT MARKINGS - TWO-LAWE AND MULTI-LANE ROADWAYS 1205.01 RALED PAVEMENT MARKERS - INSTALLATION SPACING 1250.01 RALED PAVEMENT MARKERS - INSTALLATION SPACING 1250.01 RAISED PAVEMENT MARKERS - INSTALLATION SPACING 1251.01 RAISED PAVEMENT MARKERS - INSTALLATION SPACING 1261.02 GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING 1261.02 GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING 1262.01 GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING 1262.01 GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING | SNOWPLOWABLE B) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LI C) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKER D) PASSING ZONES WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED THE ENGINEER. E) THE BACKGROUND FOR TYPE E & F SIGNS SHALL BE TYPE C REFLECTIVE SHEIR ITEM NO. ITEM NO. ITEM DESCRIPTION ONTRACTOR FURNISHED, TYPE E SIGN AUXIMACTOR FURNISHED, TYPE E SIGN AUXIMARY OF QUANTITIES ITEM NO. ITEM DESCRIPTION DESC. NO. ONTRACTOR FURNISHED, TYPE E SIGN AUXIMACTOR FURNISHED, TYPE E SIGN AUXIMINATION DESC. NO. ONTRACTOR FURNISHED, TYPE E SIGN SUPPORTS, 3 LB STEEL U-CHANNEL SUPPORTS, 3 LB STEEL U-CHANNEL AUXIMENT MARKING LINES (4*, 90 MILS) GENERIC PAVEMENT MARKING LINES (4*, 90 MILS) GENERIC PAVEMENT MARKING ITEM (SNOWPLOWABLE MARKER ALTERNATIVE) | INES. RS. BY ETING. QUANTITY UNIT 12 S.F. 28 L.F. 28 L.F. 28 L.F. 28 L.F. 28 L.F. 21 EA. 2550 L.F. 23 EA. |
| CONTRACT | PLAN SUBMITTED TO: N.C.D.O.T. SIGNING AND DELINEATION UNIT SIGNING & DELINEATION STANDARDS ENGINEER SIGNING & DELINEATION DEDIECT DESTON ENGINEER | PLAN PREPARED BY: HDR ENGINEERING, INC. OF THE CAROLINAS MIKE RZEPKA, P.E. SIGNING & DELINEATION PROJECT DESIGN ENGINEER CHBIS HARNDEN SIGNING & DELINEATION PROJECT DESIGN ENGINEER | HDR Eng 555 Fayette N.C.B.E. |

| QUANIIIES | | |
|-------------------------------------|----------|------|
| ESCRIPTION | QUANTITY | UNIT |
| | | |
| SIGN | 12 | S.F. |
| | 28 | L.F. |
| | 2 | EA. |
| NG LINES (4", 90 MILS) | 2550 | L.F. |
| I (SNOWPLOWABLE MARKER ALTERNATIVE) | 23 | EA. |

SHEET NO. PROJECT NO. HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116 **F** S&PMP-2 DF18203.2010800 APPROVED: <u>Michael T. Rzepka</u> DATE: <u>10/24/2024</u> SEAL SEAL 15876 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TIE TO EXISTING \neg EX 🛉 ____ ΕX T11-T12 401 PLACE SIGN #401 870' IN ADVANCE OF THE BRIDGE, OR AS DIRECTED BY THE ENGINEER. (SEE GENERAL NOTE "D") SIGNING & PAVEMENT MARKING PLAN

NOTE: EMBANKMENT COLUMN DOES NOT INCLUDE BACKFILL FOR UNDERCUT

| Station | Uncl. Exc. | Embt | | |
|----------|------------|-----------|--|----------|
| | | | | _ |
| L | (cu. yd.) | (cu. yd.) | | |
| 12+50.00 | 0 | 0 | | |
| 13+00.00 | 26 | 5 | | <u> </u> |
| 13+50.00 | 23 | 24 | | |
| 14+00.00 | 14 | <u> </u> | | |
| 14+05.00 | l | 12 | | |
| Station | Uncl. Exc. | Embt | | |
| L | (cu. yd.) | (cu. yd.) | | |
| 14+62.25 | 0 | 0 | | <u> </u> |
| 15+00.00 | 3 | 88 | | |
| 15+50.00 | 13 | 75 | | <u> </u> |
| 16+00.00 | 21 | 67 | | |
| 16+30.00 | 14 | 45 | | <u> </u> |
| 16+50.00 | 5 | 18 | | |
| 17+50.00 | | 21 | | |
| 17+80.00 | 0 | 2 | | |
| 17:00.07 | 0 | | | |
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STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

CROSS-SECTION SUMMARY

____ _____ _____ _____

Approximate que excavation, fin pavement will

| | PROJ. REFE | SHEET NO. | |
|---|---|---|------------------|
| | DF18203 | .2010800 | X-0 |
| uantities only. le grading, clea be paid for at t | Unclassified ex ring and grubbin he lump sum prio | cavation, borrov Ig, and removal ce for "Grading" | v of existing |
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>LOT DRIVER: NCDOT STRUCTURES DEFAULT PLOTTER.pit PENTABLE: NCDOT STRUCTURES DEF JSER: DCHAPMAN DATE: 10/21/2024 TIME: 4:35:08 PM

ST NCDOT :12 PM PENTABLE: 1 TIME: 4:35: + OTTE

| | | | | | | | Driven Piles | | | Predrilling for Piles ** | | Dr | illed-In Piles | |
|--|-----------------------------------|--|--|--|--------------------------------------|--|---|--------------------------------------|---|--|--|---|---|---|
| End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5") | Number of Piles per Line | Factored Resistance per Pile KIPS | Pile Cut-Off (Top of Pile) Elevation FT | Estimated Pile Length per Pile FT | Scour Critical Elevation FT | Minimum Pile Tip (Tip No Higher Than) Elevation FT | Required Driving Resistance (RDR)* per pile KIPS | Pile Redrives Quantity EACH | Predrilling Length per Pile LIN FT | Predrilling Elevation (Elevation Not To Predrill Below) FT | Maximum Predrilling Diameter INCHES | Pile Excavation (Bottom of Hole) Elevation FT | Pile Excavation Not In Soil per Pile LIN FT | Pile Excavation In Soil per Pile LIN FT |
| End Bent 1, Piles 1-7 | 7 | 145 | 6.30 | 45 | | | 195 | | | | | | | |
| End Bent 2, Piles 1-7 | 7 | 145 | 6.50 | 45 | | | 195 | | | | | | | |
| | | | | | | | | | | | | | | |
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| | | | | | | | | | | | | | | |
| TOTAL QUANTITY | : | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |

Factored Resistance + Factored Drag Load + Factored Dead Load + Nominal Drag Load Resistance + Nominal Resistance from Scourable Material * RDR =Dynamic Resistance Factor

** Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

PILE DESIGN INFORMATION

(Blank entries indicate item is not applicable to structure)

| End Bent / Bent No, Pile(s) #(-#) (e.g., "Bent 1, Piles 1-5") | Factored Axial Load per Pile KIPS | Factored Drag Load per Pile KIPS | Factored Dead Load * per Pile KIPS | Dynamic Resistance Factor | Nominal Drag Resistance per Pile KIPS | Nominal Scour Resistance per Pile KIPS |
|---|---|--|--|---------------------------------|---|--|
| End Bent 1, Piles 1-7 | 145 | 0 | 0 | 0.75 | 0 | 0 |
| End Bent 2, Piles 1-7 | 145 | 0 | 0 | 0.75 | 0 | 0 |
| | | | | | | |
| | | | | | | |
| | | | | | | |

* Factored Dead Load is factored weight of pile above the ground line.

NOTES:

1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Saket Kabra, #053059) on 10-17-2024.

2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.

3. The Engineer may adjust the quantity for DPT Testing when necessary.

SUMMARY OF PILE INFORMATION/INSTALLATION

(Blank entries indicate item is not applicable to structure)

SUMMARY OF DPT/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

| Dynamic Pile Testing (DPT) | | | | | | |
|---|----------------------------------|------------------------------------|--|--|--|--|
| End Bent / Bent No (e.g., "Bent 1 - Bent 3") | DPT Test Pile Length FT | DPT Testing Quantity EACH | | | | |
| End Bent 1 - End Bent 2 | 45 | 2 | | | | |
| | | | | | | |
| | | | | | | |

| Pile Order Lengths for Concrete Piles | | | | | | |
|---|---|--|--|--|--|--|
| End Bent / Bent No (e.g., "Bent 1 - Bent 3") | Pile Order Length Basis* EST or DPT | | | | | |
| End Bent 1 - End Bent 2 | DPT | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| EST = Pile order lengths from estimated pile | | | | | | |

lengths; DPT = Pile order lengths based on Dynamic Pile Testing. For groups of end bents/bents with pile order lengths based on DPT testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the DPT.

PROJECT NO. <u>DF18203.2010800</u>

BRUNSWICK

COUNTY

STATION:

14+33.62 -L-

SHEET 3 OF 4

| RTH CARO | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | | | | | |
|--|--|-----|-------|-----|-----|-------|--------|--|--|--|
| SEAL 033723 A. ABER Charles & abernatur 10/21/2024 | PILE FOUNDATION TABLES | | | | | | | | | |
| SIGNATURE DATE | REVISIONS SHEET NO. S-03 | | | | | | | | | |
| DOCUMENT NOT CONSIDERED | NO. | BY: | DATE: | NO. | BY: | DATE: | TOTAL | | | |
| FINAL UNLESS ALL | 1 | | | 3 | | | SHEETS | | | |
| SIGNATURES COMPLETED | 2 | | | 4 | | | 17 | | | |

| | | | | | | TOTAL BILL | | - A I | | | | | | |
|----------------|------------|--------------|----------|----------|-------------|---------------------|-----------------|---------|----------|---------------|------------|-------------|------------------------|------------|
| | | 1 | 1 | 1 | 1 | TOTAL DILL | | | 1 | 1 | 1 | 1 | 1 | 1 |
| | REMOVAL OF | UNCLASSIFIED | CLASS AA | BRIDGE | EPOXY | PILE DRIVING | 16" PRESTRESSED | DYNAMIC | VERTICAL | RIP RAP | GEOTEXTILE | ELASTOMERIC | $3'-0'' \times 1'-9''$ | PZ35 STEEL |
| | EXISTING | STRUCTURE | CONCRETE | APPROACH | COATED | EQUIPMENT SETUP | CONCRETE | PILE | CONCRETE | CLASS II | FOR | BEARINGS | PRESTRESSED | SHEET PILE |
| | STRUCTURE | EXCAVATION | | SLABS | REINFORCING | FOR 16" PRESTRESSED | PILES | TESTING | BARRIER | (2'-0" THICK) | DRAINAGE | | CONCRETE | SYSTEM |
| | | | | | STEEL | CONCRETE PILES | | | RAIL | | | | CORED SLABS | |
| | LUMP SUM | LUMP SUM | CU.YDS. | LUMP SUM | LBS. | EA. | NO. LIN.FT. | EACH | LIN.FT. | TONS | SQ.YDS. | LUMP SUM | NO. LIN.FT. | SQ.FT. |
| SUPERSTRUCTURE | | | | | | | | | 110.25 | | | | 11 605 | |
| END BENT 1 | | | 37.5 | | 4824 | 7 | 7 315 | | | 242 | 269 | | | 3216 |
| END BENT 2 | | | 53.1 | | 7022 | 7 | 7 315 | | | 307 | 341 | | | 4896 |
| TOTAL | LUMP SUM | LUMP SUM | 90.6 | LUMP SUM | 11846 | 14 | 14 630 | 2 | 110.25 | 549 | 610 | LUMP SUM | 11 605 | 8112 |

| DES BY: | R.JONES | DATE: 10/24 | DWG BY: _B.PETERSON | DATE :10/24 |
|----------|--------------|--------------|----------------------|-------------|
| DES CHK: | A. ABERNATHY | DATE : 10/24 | CHK BY: A. ABERNATHY | DATE :10/24 |

| 1 | | |
|---|-------------|--------------------|
| | SAM REPL | PLE BAR ACEMENT |
| | SIZE | LENGTH |
| | #3 | 6'-2" |
| | #4 | 7'-4" |
| | #5 | 8'-6" |
| | #6 | 9'-8" |
| | #7 | 10'-10" |
| | #8 | 12'-0" |
| | #9 | 13'-2" |
| | #10 | 14'-6" |
| | #11 | 15'-10" |

NOTE:

SAMPLE BAR REPLACEMENT LENGTHS BASED ON 30" (SAMPLE LENGTH) PLUS TWO SPLICE LENGTHS AND f_y = 60ksi.

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS STRUCTURE CONTAINS THE NECESSARY CORROSION PROTECTION REQUIRED FOR A CORROSIVE SITE.

FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS. FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS

ALL METALIZED SURFACES SHALL RECEIVE A SEAL COATING AS SPECIFIED IN THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE END BENT CAPS AND SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL BAR SUPPORTS USED IN THE BARRIER RAIL, END BENT CAPS AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONCRETE IN THE END BENT CAPS AND PRESTRESSED CONCRETE PILES OF END BENT NOS.1 & 2 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB OF CEMENT. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

THE 100YR THEORETICAL SCOUR RANGES FROM 1' TO 16' (CLEAR-WATER TO LIVE-BED). SIGNIFICANT SCOUR WAS OBSERVED AFTER THE STORM THAT WASHED OUT THE ROAD AND CULVERT. THE STORM SCOUR RANGES FROM EL. -5.0 TO EL.-13.0. IT IS RECOMMENDED TO CONSIDER THE STORM SCOUR (EL. -13.0) OR GREATER, FOR SCOUR CONSIDERATIONS THERE HAVE BEEN NO PRIOR HISTORICAL SCOUR ISSUES REPORTED BY NCDOT.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE SAMPLE BARS SHOULD COME FROM STEEL ACTUALLY USED IN THE PROJECT AND THE SAMPLE BARS SHOULD BE REPLACED BY SPLICED BARS AS SPECIFIED IN THE SAMPLE BAR REPLACEMENT CHART. PAYMENT FOR THE SAMPLE BARS AND REPLACEMENT REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 22 FT EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. DF18203.2010800

BRUNSWICK COUNTY

STATION: 14+33.50 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING FOR BRIDGE ON SR 1528 (E. MOORE ST.)

OVER PRICE CREEK BETWEEN NC HWY 211 AND SR 1527 (E.LEONARD ST.)

| A REAL OP | TH CARO | 1 |
|-----------|----------------|-------------------------|
| C Cł | SEAL 033723 | ~~~ |
| TR.K | AGINEE S | AN P |
| ·***** | 4. AD | ** * /71/2024 |

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| | | SHEET | | | | |
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| N0. | BY: | DATE: | NO. | BY: | 5-04 | |
| 1 | | | S | | | TOTAL |
| 2 | | | 4 | | | 17 |

| | | L | OAD AND | RESIS | STANCI | E FACTO | | TING | (LRFF | R) SL | JMMA | RY FOF | R PRE | STR | ESS | ED CO | ONCRE | TE GI | RDERS | ; | | | | |
|--------|------------|-------------------|----------------------|--------------------------------|-----------------------------------|---------------|-----------------------------|------------------------------|---------------|-------|--------------------|---|------------------------------|---------------|-------|--------------------|---|-----------------------------|------------------------------|---------------|-----------|--------------------|---|--------------|
| | | | | | | | | | | | STR | ENGTHI | LIMIT S | ΤΑΤΕ | | | | | | SERVI | CE III LI | IMIT STA | TE | |
| | | | | # | | | | | N | 10ME | NT | | | S | SHEAF | ٦ | | | | M | OMENT | - | | |
| | | | | | | | | | | | | | | | | | | - | | | | | | ER |
| | | VEHICLE | WEIGHT (W) (TONS) | CONTROLLING LOAD RATING | MINIMUM RATING FACTORS (RF) | TONS = W x RF | LIVE-LOAD FACTORS (Y LL) | DISTRIBUTION FACTORS (DF) | RATING FACTOR | SPAN | GIRDER LOCATION | DISTANCE FROM LEFT END OF SPAN (ft) | DISTRIBUTION FACTORS (DF) | RATING FACTOR | SPAN | GIRDER LOCATION | DISTANCE FROM LEFT END OF SPAN (ft) | LIVE-LOAD FACTORS (7 LL) | DISTRIBUTION FACTORS (DF) | RATING FACTOR | SPAN | GIRDER LOCATION | DISTANCE FROM LEFT END OF SPAN (ft) | COMMENT NUMB |
| | | HL-93 (INVENTORY) | N/A | | 1.055 | | 1.75 | 0.275 | 1.23 | 55' | EL | 27 | 0.523 | 1.23 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.05 | 55' | EL | 27 | |
| DESI | GN | HL-93 (OPERATING) | N/A | | 1.591 | | 1.35 | 0.275 | 1.59 | 55' | EL | 27 | 0.523 | 1.59 | 55' | EL | 5.4 | N/A | | | | | | |
| LOA | D | HS-20 (INVENTORY) | 36.000 | 2 | 1.322 | 47.585 | 1.75 | 0.275 | 1.54 | 55' | EL | 27 | 0.523 | 1.47 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.32 | 55' | EL | 27 | ļ |
| | | HS-20 (OPERATING) | 36.000 | | 1.900 | 68.396 | 1.35 | 0.275 | 1.99 | 55' | EL | 27 | 0.523 | 1.90 | 55' | EL | 5.4 | N/A | | | | | | ļ |
| | | SNSH | 13.500 | | 2.776 | 37.476 | 1.4 | 0.275 | 4.04 | 55' | EL | 27 | 0.523 | 4.17 | 55' | EL | 5.4 | 0.80 | 0.275 | 2.78 | 55' | EL | 27 | ļ |
| | Щ | SNGARBS2 | 20.000 | | 2.155 | 43.095 | 1.4 | 0.275 | 3.14 | 55' | EL | 27 | 0.523 | 3.02 | 55' | EL | 5.4 | 0.80 | 0.275 | 2.15 | 55' | EL | 27 | ļ |
| | | SNAGRIS2 | 22.000 | | 2.079 | 45.734 | 1.4 | 0.275 | 3.03 | 55' | EL | 27 | 0.523 | 2.83 | 55' | EL | 5.4 | 0.80 | 0.275 | 2.08 | 55' | EL | 27 | ļ |
| | N () | SNCOTTS3 | 27.250 | | 1.384 | 37.708 | 1.4 | 0.275 | 2.01 | 55' | EL | 27 | 0.523 | 2.09 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.38 | 55' | EL | 27 | ļ |
| | | SNAGGRS4 | 34.925 | | 1.189 | 41.527 | 1.4 | 0.275 | 1.73 | 55' | EL | 27 | 0.523 | 1.77 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.19 | 55' | EL | 27 | ļ |
| | | SNS5A | 35.550 | | 1.160 | 41.255 | 1.4 | 0.275 | 1.69 | 55' | EL | 27 | 0.523 | 1.82 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.16 | 55' | EL | 27 | |
| | | SNS6A | 39.950 | | 1.079 | 43.102 | 1.4 | 0.275 | 1.57 | 55' | EL | 27 | 0.523 | 1.68 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.08 | 55' | EL | 27 | |
| LEGAL | | SNS7B | 42.000 | | 1.028 | 43.175 | 1.4 | 0.275 | 1.50 | 55' | EL | 27 | 0.523 | 1.67 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.03 | 55' | EL | 27 | |
| LOAD | | TNAGRIT3 | 33.000 | | 1.320 | 43.556 | 1.4 | 0.275 | 1.92 | 55' | EL | 27 | 0.523 | 1.98 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.32 | 55' | EL | 27 | |
| | ۲ ۲ | TNT4A | 33.075 | | 1.330 | 43.979 | 1.4 | 0.275 | 1.94 | 55' | EL | 27 | 0.523 | 1.91 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.33 | 55' | EL | 27 | |
| | LER | TNT6A | 41.600 | | 1.101 | 45.811 | 1.4 | 0.275 | 1.60 | 55' | EL | 27 | 0.523 | 1.83 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.10 | 55' | EL | 27 | |
| | RAI ST) | TNT7A | 42.000 | | 1.114 | 46.804 | 1.4 | 0.275 | 1.62 | 55' | EL | 27 | 0.523 | 1.71 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.11 | 55' | EL | 27 | |
| | | TNT7B | 42.000 | | 1.163 | 48.848 | 1.4 | 0.275 | 1.69 | 55' | EL | 27 | 0.523 | 1.62 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.16 | 55' | EL | 27 | |
| | SEN | TNAGRIT4 | 43.000 | | 1.101 | 47.330 | 1.4 | 0.275 | 1.60 | 55' | EL | 27 | 0.523 | 1.56 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.10 | 55' | EL | 27 | |
| | | TNAGT5A | 45.000 | | 1.031 | 46.405 | 1.4 | 0.275 | 1.50 | 55' | EL | 27 | 0.523 | 1.58 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.03 | 55' | EL | 27 | |
| | | TNAGT5B | 45.000 | $\langle 3 \rangle$ | 1.013 | 45.582 | 1.4 | 0.275 | 1.47 | 55' | EL | 27 | 0.523 | 1.48 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.01 | 55' | EL | 27 | |
| EMERG | ENCY | EV2 | 28.750 | | 1.617 | 46.483 | 1.3 | 0.275 | 2.37 | 55' | EL | 27 | 0.523 | 2.27 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.62 | 55' | EL | 27 | |
| VEHICL | E (EV) | EV3 | 43.000 | $\overline{\langle 4 \rangle}$ | 1.049 | 45.107 | 1.3 | 0.275 | 1.54 | 55' | EL | 27 | 0.523 | 1.53 | 55' | EL | 5.4 | 0.80 | 0.275 | 1.05 | 55' | EL | 27 | |

ST PENTABLE: NCDOT S TIME: 4:35:24 PM + Lotter.pit DR

| DES BY: R. JONES | DATE : <u>10/24</u> | DWG BY: _ B. PETERSON | DATE : 10/24 |
|----------------------|---------------------|-----------------------|---------------------|
| DES CHK:A. ABERNATHY | DATE : <u>10/24</u> | CHK BY:A.ABERNATHY | DATE : <u>10/24</u> |

 $\begin{array}{c}
1\\
2\\
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3\\
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4
\end{array}$

LRFR SUMMARY (FOR SPAN A)

LOAD FACTORS:

| DESIGN | LIMIT STATE | γDC | γDW |
|----------------|-------------|------|------|
| LOAD RATING | STRENGTH I | 1.25 | 1.50 |
| FACTORS | SERVICE III | 1.00 | 1.00 |

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

| (#) CONTROLLING LOAD RATING | | | | | | |
|------------------------------------|--|--|--|--|--|--|
| 1 DESIGN LOAD RATING (HL-93) | | | | | | |
| 2 DESIGN LOAD RATING (HS-20) | | | | | | |
| 3 LEGAL LOAD RATING * * | | | | | | |
| 4 EMERGENCY VEHICLE LOAD RATING ** | | | | | | |
| * * SEE CHART FOR VEHICLE TYPE | | | | | | |
| GIRDER LOCATION | | | | | | |
| I - INTERIOR GIRDER | | | | | | |
| EL - EXTERIOR LEFT GIRDER | | | | | | |
| ER- EXTERIOR RIGHT GIRDER | | | | | | |

PROJECT NO. DF18203.2010800

BRUNSWICK COUNTY

STATION: 14+33.50 -L-

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| (NON-INTERSTATE TRAFFIC) | | | | | | | | | |
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| | | | SHEET | | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-05 | | | |
| ſ | | | I | | | TOTAL | | | |
| 2 4 ^{SHEETS} 17 | | | | | | | | | |
| STD. NO. 21LRFR1_90S_55L | | | | | | | | | |

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

LRFR SUMMARY FOR 55' CORED SLAB UNIT 90° SKEW

STANDARD

ц С

>LOTTER.plt PENTABLE: NCDOT S' TIME: 4:35:32 PM

+

NCDOT STRUCTURES DEFAULT PLOTTE

| 18'-4" | 18'-4" |
|---|---|
| E GROUTED ESS DETAILS (TYP.) | 10-#5 B14 IN VERTICAL CONCRETE BARRIER RAIL |
| UTTERLINE | # # # # # 5 S3 & # 5 S4 # 5 S4 |
| | |
| | |
| 12'' Ø VOIDS (TYP.EA.SLAB UNIT) | 4" (TYP.) |
| | |
| 1'-9" SPLICE | |
| | 1 |
| | |
| 0.6″ØL.R. TRANSVERSE T-TENSIONING STRAND N 2½″ØHOLE (TYP.) | |
| | |
| GUTTERLINE | |
| € '/₂'' EXP. JT. MAT'L. IN RAIL (TYP.) | #4 S2 +4 S2 +4 S2 VERTICAL CONCRETE BARRIER RAIL |
| PAIRS (SPACED AS SHOWN IN DETAIL ``A'') (TYP.EA.UNIT) | |
| 3 (SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.UNIT) PACED TO MATCH S3 IN VERTICAL CONCRETE BARRIER RA | IL) 27'-6″ |
| 55'-0" | 21-0 |
| PLAN OF UNIT | |
| | |

| R VERTICAL CONCRETE BARRIER RAIL | | | | | | | | |
|----------------------------------|-----------|------|------|--------|--------|--|--|--|
| IOR UNITS | TOTAL NO. | SIZE | TYPE | LENGTH | WEIGHT | | | |
| | | | | | | | | |
| | | | | | | | | |
| | 40 | #5 | STR | 27'-1" | 1130 | | | |
| | | | | | | | | |
| | 128 | #5 | 2 | 7'-2″ | 957 | | | |
| | | | | | | | | |
| EL | | | LBS. | | 2087 | | | |
| CU.YDS. 14.1 | | | | | | | | |
| R RAIL LN.FT. 110.25 | | | | | | | | |
| | | | | | | | | |

| | BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT | | | | | | | |
|----------------------------|---|----------|--------|---------|--------|---------|--|--|
| | | | EXTERI | OR UNIT | INTERI | OR UNIT | | |
| BER | SIZE | TYPE | LENGTH | WEIGHT | LENGTH | WEIGHT | | |
| | #4 | STR | 28′-3″ | 75 | 28'-3" | 75 | | |
| | | | | | | | | |
| | #5 | 3 | 4'-3" | 35 | 4'-3" | 35 | | |
| | #4 | 3 | 5′-4″ | 406 | 5′-4″ | 406 | | |
| | #5 | 1 | 5′-7″ | 373 | | | | |
| | | | | | | | | |
| | | | | | | | | |
| IG STEEL LBS. | | . | 516 | | 516 | | | |
| ATED ING STEEL LBS. 373 | | | | | | | | |
| CONCRETE CU. YDS. | | 7.8 | | 7.8 | | | | |
| | | | | | | | | |
| STR | ANDS | Nc |) _ | 19 | | 19 | | |
| | | | | | | | | |

| CORED | SLABS | s req | UIRED |
|---------------|--------|--------|--------------|
| | NUMBER | LENGTH | TOTAL LENGTH |
| 55′UNIT | | | |
| EXTERIOR C.S. | 2 | 55'-0" | 110'-0" |
| INTERIOR C.S. | 9 | 55'-0" | 495′-0″ |
| TOTAL | 11 | | 605'-0" |

| GUTTERLINE ASP | HALT THICKNESS & RAI | L HEIGHT |
|----------------|---------------------------|-------------|
| | ASPHALT OVERLAY THICKNESS | RAIL HEIGHT |
| | @ MID-SPAN | @ MID-SPAN |
| 55' UNIT | 15⁄8″ | 3′-75⁄8″ |

| GRADE 270 S | TRANDS |
|---------------------------------------|-----------|
| | 0.6″ØL.R. |
| AREA (SQUARE INCHES) | 0.217 |
| ULTIMATE STRENGTH (LBS.PER STRAND) | 58,600 |
| APPLIED PRESTRESS (LBS.PER STRAND) | 43,950 |

| DEAD LOAD DEFLECTION AN | ND CAMBER |
|---|-----------------------------|
| | 3'-0"× 1'-9" |
| 55' CORED SLAB UNIT | 0.6″ØL.R. STRAND |
| CAMBER (SLAB ALONE IN PLACE) | 1¹⁄₂″ ♦ |
| DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD | 3∕8″ ↓ |
| FINAL CAMBER | 1 ∕8″ ♦ |

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST. AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " in depth, shall be tooled in all EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR. SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCLIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PROJECT NO. DF18203.2010800

BRUNSWICK COUNTY

STATION: 14+33.50 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

3'-0" X 1'-9" PRESTR. CONC. CORED SLAB UNIT 90° SKEW DETAILS

| | | REVIS | SIO | NS | | SHEET |
|-----|-----|-------|------------|-----|-------|-------|
| N0. | BY: | DATE: | NO. | BY: | DATE: | S-08 |
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| 2 | | | 4 3 | | | 17 |

CONCRETE RELEASE STRENGTH

| PSI |
|------|
| 4900 |
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE

PENTABLE: | TIME: 4:35:

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

END OF CORED SLAB UNIT @ END BENT NO.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - ⁷/₈" Ø BOLTS WITH NUTS AND WASHERS.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE 1 ¹/₄" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

| 1 * * | * END OF CORED SLAB WNIT @ END BENT No. 2 |
|---|--|
| SKETCH SHOWIN POINTS OF ATTACH * DENOTES GUARDRAIL ANCHOR | IG MENT ASSEMBLY |
| PF S1 | OJECT NO. <u>DF18203.2010800</u> BRUNSWICK COUNTY ATION: 14+33.50 -L- |
| BEAL OROFESSION SEAL OBJECT SEAL OBJECT SEAL OBJECT A. ABERTING Unarles & abernatury | DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL ONCRETE BARRIER RAIL |
| ne Carolinas gh, N.C. 27601 ber: F-0116 UNLESS ALL SIGNATURES COMPLETED | BY: DATE: No. BY: DATE: No. 3 TOTAL SHEETS 17 |

ST NCDOT :44 PM PENTABLE: TIME: 4:35:

HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

THE CONCRETE IN THE END BENT CAPS SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THE THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. ANCHOR PLATES, WASHERS, AND NUTS SHALL MEET THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS. ANCHOR BOLTS, ANCHOR PLATES. WASHERS. AND NUTS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

NO SEPARATE PAYMENT SHALL BE MADE FOR THE ANCHOR BOLTS, ANCHOR PLATES, WASHERS, AND NUTS. THE COST OF THE MATERIAL AND INSTALLATION SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

SHEET PILES SHALL BE METALLIZED.

FOR SHEET PILE WALL AND WING DETAILS, SEE SHEET 3 OF 4.

PROJECT NO. DF18203.2010800

BRUNSWICK COUNTY

STATION: 14+33.50 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE END BENT NO.1

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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

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NO SEPARATE PAYMENT SHALL BE MADE FOR THE ANCHOR BOLTS, ANCHOR PLATES, WASHERS. AND NUTS. THE COST OF THE MATERIAL AND INSTALLATION SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

SHEET PILES SHALL BE METALLIZED.

FOR SHEET PILE WALL AND WING DETAILS, SEE SHEET 3 OF 4.

PROJECT NO. DF18203.2010800

BRUNSWICK COUNTY

SHEET NO**.** S-11

TOTAL SHEETS 17

STATION: 14+33.50 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE END BENT NO.2

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| | | | | FOF | R ON | IE E | ND BE | INT |
| , | | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| •• | | | B1 | 8 | #9 | 1 | 40'-10" | 1111 |
| 3″ | 1'-3'' LAP | | B2 | 28 | #4 | STR | 20'-6" | 384 |
| | | | B3 | 10 | #4 | STR | 2'-9" | 19 |
| | | | 84 | 6 | #5 | SIR | 42'-6" | 266 |
| | | | D1 | 18 | #6 | STR | 1'-6" | 41 |
| | $\left(\begin{array}{c} \overline{5} \end{array}\right)$ | | | 10 | | | | |
| | | | K1 | 12 | #4 | STR | 2'-10" | 23 |
| | | | | | | | | |
| | | | S1 | 50 | #4 | 3 | 10'-5" | 349 |
| | <u>2′−2″Ø</u> | | S2 | 50 | #4 | 4 | 3'-6" | 118 |
| | | | 55 | 28 | | 2 | 8 -1 | 152 |
| | [| | V1 | 140 | #6 | STR | 5′-5″ | 1140 |
| | | 1 | | 1.0 | | | | |
| | | Ľ | H1 | 28 | #4 | 2 | 16'-11" | 317 |
| | | В | Н3 | 10 | #5 | STR | 20'-6" | 214 |
| | | EN | | - 10 | | | | 475 |
| | | | | 40 | #5 #E | 6 | 10'-5" | 435 |
| | | | 02 | 38 | #D | б | 6-5 | 200 |
| | | | V1 | 300 | #6 | STR | 5'-5" | 2441 |
| // | 11″ | | | | | _ | | |
| | | . 2 | H1 | 28 | #4 | 2 | 16'-11" | 317 |
| | | ВТ | H2 | 28 | #4 | STR | 22'-5" | 420 |
| 1 | | ND | H4 | 10 | #5 | STR | 40'-6" | 423 |
| | | ш | | 40 | #5 | 6 | 10'-5" | 435 |
| | | | U2 | 78 | #5 | 6 | 6′-5″ | 523 |
| ONS | ARE OUT TO OUT. | | EPOXY | COA | TED RE | INFOR | CING STEE | EL |
| | | | END B | ENT 1 |)) | | | 1824 LBS. |
| | END BENI NO.2 | | END E | ENI Z | - | | | UZZ LBS. |
| | PZ35 STEEL SHEET PILE SYSTEM | _ | (| CLASS | AA CC ENI | NCRET D BEN ⁻ | E BREAKD(T 1 | NWC |
| 6 | SQ.FI.: 4896 | ò | POUR | #1 C | AP.IOW | VFR PA | RT OF | 34.1 C.Y. |
| | | | 1 0 011 | Ŵ | INGS 8 | k COPI | ING | |
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| | | | (| CLASS | AA CC ENI | NCRET | E BREAKDO | NWC |
| | | | POUR | #1 C W | AP,LOW Ings 8 | VER PA & COPI | NRT OF ING | 46.0 C.Y. |
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SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE END BENT NO.1 & 2 DETAILS

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| | CONCRETE | PILE WT. | ONE POIN | IT PICK-UP | TWO POIN | T PICK-UP |
|---------|----------|----------|----------|------------|-----------------|-----------|
| LENGTH | CU. YDS. | TONS | 0.300L | 0.700L | 0.207L | 0.586L |
| 25'-0" | 1.63 | 3.31 | 7'-6" | 17'-6'' | 5'-2" | 14'-8" |
| 30'-0" | 1.96 | 3.97 | 9'-0'' | 21'-0" | 6' - 2½" | 17'-7" |
| 35'-0" | 2.29 | 4.63 | 10'-6" | 24'-6" | 7'-3" | 20'-6" |
| 40'-0'' | 2.61 | 5.29 | 12'-0'' | 28'-0" | 8'-3½" | 23'-5" |
| 45'-0'' | 2.94 | 5.95 | 13'-6" | 31'-6" | 9'-4" | 26'-4" |
| 50'-0'' | 3.27 | 6.61 | 15'-0'' | 35'-0" | 10'-4" | 29'-4" |
| 55'-0'' | 3.59 | 7.28 | 16'-6" | 38'-6" | 11'-4½" | 32'-3" |
| 60'-0'' | 3.92 | 7.94 | | | 12'-5" | 35'-2" |
| 65'-0'' | 4.25 | 8.60 | | | 13'-5½" | 38'-1" |
| 70'-0'' | 4.57 | 9.26 | | | 14'-6'' | 41'-0'' |
| 75'-0" | 4.90 | 9.92 | | | 15'-6½" | 43'-11" |
| 80'-0" | 5.23 | 10.58 | | | 16'-7" | 46'-10" |

FOR BURNING STRANDS

DOWEL INSTALLATION FOR OPTIONAL BUILD-UP

GROUT COMPRESSIVE STRENGTH: f'c= 5,000 PSI

BEFORE DRILLING DOWEL HOLES, REMOVE THE UPPER 3" OF CONCRETE FROM THE TOP OF THE PILE WITHOUT DAMAGE TO THE REINFORCING STEEL. THE REMOVAL PLANE SHOULD BE NORMAL TO THE EDGE OF THE PILE.

DOWEL HOLES SHALL BE POSITIONED TO MAINTAIN ¹/₂" CLEAR TO ALL EXISTING PRESTRESSING STRANDS IN THE CONCRETE PILE.

FIELD DRILLED HOLES SHALL BE CLEAN AND FREE OF ANY OBSTRUCTIONS BEFORE GROUTING OF DOWELS. DOWEL BARS SHALL BE INSTALLED AND GROUTED WITH AN APPROVED NON-SHRINK GROUT.

THE SPIRAL REINFORCING IN ALL BUILD-UPS SHALL BE W4.0 COLD DRAWN WIRE WHICH SHALL BE SECURED TO THE LONGITUDINAL REINFORCEMENT TO MAINTAIN PITCH.

THE SPIRAL REINFORCING IN THE BUILD-UP AND THE PRESTRESSED CONCRETE PILE SHALL BE SPLICED BY OVERLAPPING A MIN. OF ONE TURN.

NOTES

PRESTRESSED CONCRETE STRENGTH fc = 7,500 PSI BUILD-UP CONCRETE STRENGTH : f c = 7,500 PSI STRAND DATA:

| SIZE | GRADE | AREA | ULTIMATE STRENGTH | APPLIED PRESTRESS FORCE |
|------|----------|-------|-----------------------|-------------------------------|
| 0.6" | 270 L.R. | 0.217 | 58,600# PER STRAND | 43,940# PER STRAND |

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS CONFORMING TO AASHTO M203. STRAND SAMPLING REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED TRANSFER THE LOAD FROM THE ANCHORAGES TO THE PILE AFTER THE CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN OPPOSITE PAIRS AS INDICATED IN THE TYPICAL PATTERN SHOWN. FOR ANY NUMBER OF STRANDS, BURN IN OPPOSITE PAIRS AND SYMMETRICALLY ABOUT BOTH THE VERTICAL AND HORIZONTAL AXES. STRANDS 1-1 SHALL BE BURNED BEFORE 2-2, ETC. NOT MORE THAN 4 STRANDS, SAY 3-3 AND 4-4, MAY BE BURNED AT ANY ONE SECTION BEFORE THESE SAME PAIRS OF STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS ARE TO BE INDICATED WITH A 2" WIDE BLACK MARK.

DRIVE PILES USING A METHOD APPROVED BY THE ENGINEER WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED

DRIVING OF THE BUILT-UP PILE WILL NOT BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 5,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

THE WATER/CEMENT RATIO FOR CONCRETE PILES SHALL NOT EXCEED 0.40.

PRESTRESSED CONCRETE PILES SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONCRETE IN THE PRESTRESSED CONCRETE PILES SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

Charles & abernaty

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED PROJECT NO. <u>DF18203.2010</u>800

COUNTY

STATION: 14+33.50 -L-

BRUNSWICK

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

| 16" PRESTRESSED |
|-----------------|
| CONCRETE PILE |

| | | SHEET | | | | |
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| NOTES : |
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| FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING. |

| ESTIMATED QUANTITIES | | | | | | |
|-------------------------------|--------------------------------------|----------------------------|--|--|--|--|
| BRIDGE @ STA. 14+33.50 -L- | RIP RAP CLASS II (2'-0" THICK) | GEOTEXTILE FOR DRAINAGE | | | | |
| | TONS | SQUARE YARDS | | | | |
| END BENT 1 | 242 | 269 | | | | |
| END BENT 2 | 307 | 341 | | | | |

PROJECT NO. DF18203.2010800 BRUNSWICK COUNTY

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

RIP RAP DETAILS

3'-6" MIN

STATION: <u>14+33.50</u> -L-

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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SHEET NO**.** S-15 REVISIONS NO. BY: DATE: DATE: BY: TOTAL SHEETS 17 - ------ -- -

S NCDOT PENTABLE: I TIME: 4:36: +OTTER. ä

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| SPL | ICE LE | NGTHS |
|-------------|-----------------|----------|
| BAR SIZE | EPOXY COATED | UNCOATED |
| #4 | 1'-11" | 1'-7" |
| #5 | 2'-5″ | 2'-0" |
| #6 | 3'-7" | 2'-5" |

HDR Engineering, Inc. of the 555 Fayetteville St., Suite 900 Raleigh N.C.B.E.L.S. License Number

| | | | <u> </u> | | <u> </u> | | |
|--|---------------|------------------|------------------------|-----------------------------|-------------------------|---------------------------|-----------------------------|
| S | \vdash | BILL OF MATERIAL | | | | | |
| PLANS. | | AF BAR | N0. | SIZE | JLA TYPE | LENGTH | <u>יי</u> <u>WE</u> IGHT |
| CH SLAB SHALL BE GRADED TO ACE OF THE BRIDGE AND SHALL | * | € A1 A2 | 13 13 | #4 #4 | STR STR | 31'-10" 31'-10" | 276 |
| ED. | * | € B1 B2 | 64 64 | #5 #6 | STR STR | 11'-2" 11'-8" | 745 |
| | | | | | | | 1707 |
| | * | | Y COA | ATED | | LBS. | 1021 |
| | | | | | | | 10.4 |
| | | AP | PRO | ACH | <u>e</u> Slai | <u>с.</u> ч. В АТ Е | B #2 |
| 7-1- | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| | | A2 | 13 | #4 | STR | 31'-10" | 276 |
| CRADE TOM OT | * | € B1 B2 | 64 64 | #5 #6 | STR STR | 11'-2" 11'-8" | 745 1121 |
| | R | REINFO | DRCIN | G STEE | L | LBS. | 1397 |
| CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL | * | € EPOX REIN | Y COA | ATED Eng st | EEL | LBS. | 1021 |
| BACKFILL EXCAVATION HOLE | C | CLASS | AA C | ONCRET | E | C.Y. | 18.4 |
| ANDE STRUCTED IMMEDIATELY OF THE END BENT EXCAVATION, BOTTOM OF THE SLOPE AND PROVIDE ERIAL, SUCH AS FIBERGLASS ROVING ENGINEER TO PREVENT SOIL EROSION EA ADJACENT TO THE STRUCTURE. E REQUIRED TO REMOVE THESE NSTRUCTION OF THE APPROACH SLAB. AINAGE DETAIL | | | | | | | |
| R ◀ ┨ | | – ELB | ЭW | | | | |
| ROL IN 2'-O'MIN. S MIN. FUTURE SHOULDER | 4'-0" | | | EMPORA OPE AIN ELI | RY BOW | | |
| | | CLASS FOR E | -``B″S ROSIC TIO | N CON | TROL | | |
| | (2″MIN. — | | 3" ERC MATER | OSION RIAL O | RESIS VER P EARTH | TANT IPE I DITCH BL | _OCK |
| ZZZ EROSION RESISTANT MATERIAL | | | / | | ; | <u> </u> | |
| CONSTRUCTION OF THE APPROACH SLAB, PROVIDE TEMPORARY BERM AND SLOPE | | | | | | X | |
| L GRADE TO PIPE INLET ESISTANT MATERIAL AS SHOWN, THE ERIAL SHALL BE ETTHER 1) ASPHALT | | 4' | -0″ MI | N. | | | |
| (PE 2, MIN. 2" DEPTH, 2) EROSION CONTROL DIRECTED BY THE ENGINEER. | | | | | ۲ F | FILL SLOP | E |
| CONSIST OF A NON-PERFORATED PE,12 INCHES IN DIAMETER. | | <u>SE</u> | CTI | ON S | <u>5-S</u> | | |
| VIEW | | - | - | - - - - | - | | |
| TO BE USED WHEN SHOULDED DEDMA OUT | L DRA | | <u>D</u> | <u>- A</u> | ILS | <u> </u> | |
| UIU DE USED WHEN SHUULDEK BERM GUI | IER IS KEQ | UIKEL | J I | | | | |
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| BRUNSWICK COUNTY | | | | | | UNTY | |
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| COFESSION - 1 | DRID A | NR | A PR | F F S | NUA FR | UN C NNA | DLAD C |
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| The CINE FILL ABERTY AND A ABORTY AND A ABOR | | - · · L | | | | | |
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| UNLESS ALL SIGNATURES COMPLETED | 2 | | 4 | 왕 | | | 17 |

STD.NO.BAS_33_90S

| SPECIFICATIONS | AASHTO (CURRENT) | | |
|---|----------------------------------|--|--|
| LIVE LOAD | SEE PLANS | | |
| IMPACT ALLOWANCE | SEE AASHTO | | |
| STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 | 20,000 LBS. PER SQ. IN. | | |
| - AASHTO M270 GRADE 50W | 27,000 LBS. PER SQ. IN. | | |
| - AASHTO M270 GRADE 50 | 27,000 LBS. PER SQ. IN. | | |
| REINFORCING STEEL IN TENSION - GRADE 60 | 24,000 LBS. PER SQ. IN. | | |
| CONCRETE IN COMPRESSION | 1,200 LBS. PER SQ. IN. | | |
| CONCRETE IN SHEAR | SEE AASHTO | | |
| STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS | 1,800 LBS. PER SQ. IN. | | |
| COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER | 375 LBS. PER SQ. IN. | | |
| EQUIVALENT FLUID PRESSURE OF EARTH | 30 LBS. PER CU. FT. (MINIMUM) | | |

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1½" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION. SETTLEMENT. ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS. VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES. DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ "Ø STUDS FOR 4 - ³/₄" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

PROJECT NO. <u>DF18203.201</u>0800

BRUNSWICK COUNTY

STATION: 14+33.50 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD NOTES

| REVISIONS | | | | | | SHEET | |
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